

Wind Load Calculation Package

MWFRS design wind pressures · 60 × 120 × 52 ft · V = 115 mph · Exposure C

PRELIMINARY – NOT FOR CONSTRUCTION

Preliminary – for review by the Engineer of Record. Not for construction until reviewed and sealed by a licensed professional engineer.

Design Criteria

Wind loads are determined per ASCE 7-16 for the Main Wind Force Resisting System. The building measures 60 ft by 120 ft in plan with a mean roof height of 52 ft. The site basic wind speed is 115 mph (3-second gust) with Exposure C. The building is classified as enclosed. The governing velocity pressure at the mean roof height is $q_h = 31.74$ psf, applied with a gust-effect factor $G = 0.85$ and an internal pressure coefficient (GC_{pi}) = ± 0.18 .

Governing q_h
31.74 psf

Gust factor G
0.850

(GC_{pi})
 ± 0.18

Enclosure
enclosed

Load Derivation

Each quantity below is a pure function of its inputs, shown with the clause it comes from and the formula with values substituted in.

Velocity pressure exposure coefficient

[ASCE 7 7-16 §26.10.1, Table 26.10-1](#)

$$K_z = 2.01 \cdot (z / z_g)^{(2/\alpha)}$$
$$K_z = 2.01 \cdot (52.0 / 900)^{(2/9.5)} = 1.103$$

Topographic factor

[ASCE 7 7-16 §26.8.2, Eq. 26.8-1](#)

$$K_{zt} = 1.0 \text{ where the §26.8.1 speed-up conditions are not met}$$
$$K_{zt} = 1.000$$

Wind directionality factor

[ASCE 7 7-16 §26.6, Table 26.6-1](#)

$$K_d \text{ from Table 26.6-1}$$
$$K_d = 0.85 \text{ (buildingMwfrs)}$$

Ground elevation factor

ASCE 7 7-16 §26.9, Eq. 26.9-1

$$K_e = e^{(-0.0000362 \cdot z_g)}$$
$$K_e = e^{(-0.0000362 \cdot 0)} = 1.000$$

Velocity pressure

ASCE 7 7-16 §26.10.2, Eq. 26.10-1

$$q_z = 0.00256 \cdot K_z \cdot K_{zt} \cdot K_d \cdot K_e \cdot V^2$$
$$q_z = 0.00256 \cdot 1.103 \cdot 1.000 \cdot 0.850 \cdot 1.000 \cdot 115^2 = 31.737 \text{ psf}$$

Gust-effect factor (rigid building)

ASCE 7 7-16 §26.11.1

$$G = 0.85 \text{ for a rigid building (§26.11.1)}$$
$$G = 0.85$$

Internal pressure coefficient

ASCE 7 7-16 §26.13.1, Table 26.13-1

$$(GC_{pi}) = \pm \text{ value from Table 26.13-1, applied with both signs}$$
$$(GC_{pi}) = \pm 0.18 \text{ (enclosed)}$$

Design Wind Pressures

Wind along X (L = 60 ft, B = 120 ft)

Surface	z	q (psf)	C _p	p +(GC _{pi})	p -(GC _{pi})
windward	15 ft	24.43	0.80	10.90	22.32
windward	30 ft	28.27	0.80	13.51	24.93
windward	45 ft	30.79	0.80	15.22	26.65
windward	52 ft	31.74	0.80	15.87	27.29
leeward	h	31.74	-0.50	-19.20	-7.78
side	h	31.74	-0.70	-24.60	-13.17

Wind along Y (L = 120 ft, B = 60 ft)

Surface	z	q (psf)	C _p	p +(GC _{pi})	p -(GC _{pi})
windward	15 ft	24.43	0.80	10.90	22.32
windward	30 ft	28.27	0.80	13.51	24.93
windward	45 ft	30.79	0.80	15.22	26.65
windward	52 ft	31.74	0.80	15.87	27.29

Surface	z	q (psf)	Cp	p +(GCpi)	p -(GCpi)
leeward	h	31.74	-0.30	-13.81	-2.38
side	h	31.74	-0.70	-24.60	-13.17

Torsional Load Cases · ASCE 7-16 Fig. 27.3-8

Case	Pressure factor	Both axes	Eccentricity (ft)
1	1.0000	no	eX=±0.00, eY=±0.00
2	0.7500	no	eX=±9.00, eY=±18.00
3	0.7500	yes	eX=±0.00, eY=±0.00
4	0.5625	yes	eX=±9.00, eY=±18.00

Items for Engineer of Record Review

Before sealing, the Engineer of Record must review the following: (1) $G = 0.85$ assumes a rigid building (fundamental frequency $n_1 \geq 1$ Hz, §26.2). The Engineer of Record must confirm rigidity; a flexible building requires G_f (§26.11.5). (2) The 0.75 pressure reduction, the 0.15 eccentricity fraction and the Case 4 combined factor are transcribed from Fig. 27.3-8 but not yet verified against the official figure (VERIFICATION_QUEUE.md). Confirm before relying on Cases 2–4. (3) Roof pressures are not included: the Fig. 27.3-1 roof coefficients are not yet transcribed (VERIFICATION_QUEUE.md). This package covers wall MWFRS pressures only. These items were flagged automatically and are not a substitute for the Engineer of Record's independent judgment.

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Narrative source: deterministic-fallback. Numeric results are computed by the StructCalc engine (pure functions); narrative text is descriptive only and is never the source of a number.